DRAWING INDEX			
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2ND FLOOR REFLECTED CEILING PLAN		x	
CONCRETE DETAILS		X	_
DETAILS		X	
DETAILS	,	-	
1			

ISSUE LOG KEY:

S-001 S-002

S-003

S-004 S-005

S-121

S-122

S-123

S-501 S-601

S-602

' X 'ISSUED AS PART OF A SET ' - ' NOT A PART OF ISSUED SET ' * ' FOR INFORMATION ONLY



LIST OF ABBREVIATIONS

A.B.	ANCHOR BOLT	GA.	GAUGE
ACI	AMERICAN CONCRETE INSTITUTE	GALV.	GALVANIZED
ADD'L.	ADDITIONAL	GL	GLULAM
AESS	ARCHITECTURALLY EXPOSED	HORIZ.	HORIZONTAL
AISC	AMERICAN INSTITUTE OF STEEL	HSS	HOLLOW STRUCTURAL STEEL
	CONSTRUCTION	IBC	INTERNATIONAL BUILDING CODE
ALT.	ALTERNATE	I.D.	INSIDE DIAMETER
ALUM.	ALUMINUM	IN.	INCHES
ARCH.	ARCHITECT / ARCHITECTURAL	INT.	INTERIOR
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	K	KIPS
ASD	ALLOWABLE STRENGTH DESIGN	KSF	KIPS PER SQUARE FOOT
ASTM	AMERICAN SOCIETY FOR	LBS.	POUNDS
	TESTING AND MATERIALS	L.L.	LIVE LOAD
AWS	AMERICAN WELDING SOCIETY	LLH	LONG LEG HORIZONTAL
BLDG.	BUILDING		
BOT.	BOTTOM		
BRBF	BUCKLING RESTRAINED BRACED		
<u> </u>		LONG.	
C.G.			
C.I.P.			
C.J.		LVL	
C.J.P.		MAX.	
	CENTERLINE	MBMA	METAL BUILDING MANUFACTURE ASSOCIATION
CLR.		MECH.	MECHANICAL
CLI		MEPF	MECHANICAL, ELECTRICAL, PLUM
CMU	CONCRETE MASONRY UNIT		AND FIRE SAFETY
COL.	COLUMN	MFR.	MANUFACTURER
CONC.	CONCRETE	MIN.	MINIMUM
CONN.	CONNECTION	MISC.	MISCELLANEOUS
CONST.	CONSTRUCTION	MPH	MILES PER HOUR
CONT.	CONTINUOUS	MPP	MASS PLYWOOD PANELS
db	BAR DIAMETER	MT	MAGNETIC PARTICLE TESTING
DBA	DEFORMED BAR ANCHOR	(N)	NEW
DET.	DETAIL	N.I.C.	NOT IN CONTRACT
DIA., Ø	DIAMETER	NLT	NAIL LAMINATED TIMBER
DIAG.	DIAGONAL	NOM.	NOMINAL
D.L.	DEAD LOAD	NO.	NUMBER
DLT	DOWEL LAMINATED TIMBER	N.T.S.	NOT TO SCALE
DWG.	DRAWING	0.C.	ON CENTER
ELEC.	ELECTRICAL	O.D.	OUTSIDE DIAMETER
EL.	ELEVATION	OPP.	OPPOSITE
EQ.	EQUAL	OSL	ORIENTED STRAND LUMBER
EXIST., (E)	EXISTING	OWJ	OPEN WEB JOIST
EXP.	EXPANSION	PAF	POWDER ACTUATED FASTENER
EXT.	EXTERIOR	PART.	PARTITION
FDN.	FOUNDATION	P/C	PRECAST
FIN.	FINISH	PCF	POUNDS PER CUBIC FOOT
FLR.	FLOOR	PERIM.	PERIMETER
FRT	FIRE RETARDANT TREATED	PL	PLATE
FT.	FOOT	PP	PARTIAL PENETRATION
FTG.	FOOTING		

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REVISIONS TO THIS SHEET REV. DATE DESCRIPTION

SET ISSUE	DATE
BP	2024-02-13
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PROJECT TRACKING			
RBA #:	2327		
P.I.C:	BJ		
PM / PA:	PK/SL		

Owner OSU FRC

Project Name AZALEA EARLY CHILDHOOD CENTER

Project Address 1050 SW MADISON AVE, CORVALLIS OR 97333

DRAWING INDEX AND LIST OF ABBREVIATIONS



	PSF	POUNDS PER SQUARE FOOT
	PSL	PARALLEL STRAND LUMBER
	PSI	POUNDS PER SQUARE INCH
	P/T	POST-TENSIONED
	P.T.	PRESSURE TREATED
Ε	PVC	POLYVINYL CHLORIDE
	R, RAD.	RADIUS
	RCSC	RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS
	REF.	REFERENCE
	RET.	RETURN
	REINF.	REINFORCING
	REQ'D.	REQUIRED
	REQ'MTS.	REQUIREMENTS
	SCHED.	SCHEDULE
	S.C.	SLIP CRITICAL
	SCL	STRUCTURAL COMPOSITE LUMBER
	SIM.	SIMILAR
	SLFS	SEISMIC FORCE RESISTING SYSTEM
	S.O.G.	SLAB ON GRADE
	SPEC.	SPECIFICATION
	SQ.	SQUARE
	SS	STAINLESS STEEL
RERS	SSMA	STEEL STUD MANUFACTURERS ASSOCIATION
	STD.	STANDARD
JMBING	STRUCT.	STRUCTURAL
	SYM.	SYMMETRICAL
	THRU	THROUGH
	T&G	TONGUE AND GROOVE
	TRANS.	TRANSVERSE
	TS	LIGHT GAUGE TUBE STEEL
	TYP.	TYPICAL
	U.N.O.	UNLESS NOTED OTHERWISE
	U.T.	ULTRASONIC TESTING
	ULT.	ULTIMATE STRENGTH DESIGN LOAD LEVEL
	VERT.	VERTICAL
	V.I.F.	VERIFY IN FIELD
	w/	WITH
	WF	WIDE FLANGE
	w/o	WITHOUT
	W.P.	WORK POINT
	WPS	WELDING PROCEDURE SPECIFICATION
	WWF	WELDED WIRE FABRIC

GENERAL

STRUCTURAL DRAWINGS ARE A PART OF THE CONTRACT DOCUMENTS AND ARE COMPLEMENTARY TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING DRAWINGS, THE SPECIFICATIONS AND OTHER CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THE CONTRACT DOCUMENTS INTO THEIR SHOP DRAWINGS AND WORK. AS REQUIRED BY THE GENERAL CONDITIONS, THE CONTRACTOR SHALL PROMPTLY REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES, OR OMISSIONS IN THE CONTRACT DOCUMENTS DISCOVERED BY OR MADE KNOWN TO THE CONTRACTOR.

THE GENERAL STRUCTURAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. WHERE CONFLICT EXISTS, THE MORE STRINGENT OR RESTRICTIVE REQUIREMENT SHALL GOVERN UNITL CLARIFICATION IS REQUESTED.

CODE REQUIREMENTS:

CONFORM TO THE 2022 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED ON THE 2021 INTERNATIONAL BUILDING CODE (IBC).

TEMPORARY CONDITIONS:

THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES UNTIL COMPLETION.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

EXCAVATIONS SHALL NOT REDUCE THE VERTICAL OR LATERAL SUPPORT FOR ANY FOUNDATION OF THIS PROJECT OR ANY ADJACENT STRUCTURE WITHOUT FIRST UNDERPINNING OR PROTECTING THE FOUNDATION AGAINST DETRIMENTAL LATERAL AND/OR VERTICAL MOVEMENT.

EXISTING CONDITIONS:

ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

ASSUMED FUTURE CONSTRUCTION:

VERTICAL: NONE HORIZONTAL: NONE

DESIGN CRITERIA

DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN, WITH LIVE LOADS (L.L.) REDUCED PER OSSC:

GRAVITY SYSTEM CRITERIA				
OCCUPANCY OR USE	UNIFORM LOAD	CONCENTRATED LOAD		
ROOF LIVE/SNOW LOAD	25 PSF L.L. (ALSO SEE SNO	W LOAD CRITERIA BELOW)		
STAIRS AND LANDINGS	100 PS	SF L.L.		
	SNOW CRITERIA			
DESIGN ROOF SNOW LOAD	25 PSF MINIMUM IN ACC	CORDANCE WITH OSSC		
SNOW DRIFT	PER OSSC AS SH	IOWN ON PLANS		
GROUND SNOW LOAD	Pg= 1 IN ACCORDANCE WI	4 PSF TH: snowload.seao.org		
FLAT ROOF SNOW LOAD	Pf = 1	1 PSF		
SNOW EXPOSURE FACTOR	Ce =	= 1.0		
SNOW LOAD IMPORTANCE FACTOR	ls =	1.0		
THERMAL FACTOR	Ct =	: 1.0		
	GEOTECHNICAL CRITERIA			
ALLOWABLE SOIL PRESSURE:	1500 PSF PER OS	SC TABLE 1806.3		
WIND CRITERIA				
RISK CATEGORY II				
MAIN WIND FORCE RESISTING SYSTEM	V = 96 MPH BASIC DESIGN WIND SPEED (3-SECOND GUST)			
COMPONENTS AND CLADDING	V = 96 MPH BASIC DESIGN WIND SPEED (3-SECOND GUST)			
EXPOSURE CATEGORY	В			
GUST / INTERNAL PRESSURE	GCpi = +/- 0.18			
SEISMIC CRITERIA				
RISK CATEGORY				
SEISMIC DESIGN CATEGORY	[)		
SITE CLASS	[)		
	IE =	1.00		
ACCELERATION	Ss = 0.879	S1 = 0.465		
SITE COEFFICIENT	Fa = 1.2	Fv = NA		
DESIGN SPECTRAL ACCELERATION	SDS = 0.703	SD1 = NA		
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	PER ASCE 7-16, SECTION 12.8		
	X DIRECTION (EAST / WEST)	Y DIRECTION (NORTH / SOUTH)		
SEISMIC FORCE RESISTING SYSTEM (SFRS)	EXISTING EXTERIOR LIGHT FRAMED SHEAR WALLS (NO REVISIONS)	EXISTING EXTERIOR LIGHT FRAMED SHEAR WALLS (NO REVISIONS)		

STRUCTURAL OBSERVATIONS

THE STRUCTURAL ENGINEER OF RECORD (SEOR) WILL PERFORM STRUCTURAL OBSERVATIONS BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT ADVANCED NOTICE AND ACCESS FOR THE SEOR TO PERFORM THESE OBSERVATIONS.

ITEM	COMMENTS
PRIOR TO FIRST CONCRETE POUR	AFTER REBAR PLACEMENT
DURING INITIAL WOOD FRAMING CONSTRUCTION	
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	

A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.

STRUCTURAL OBSERVATION IS FOR THE GENERAL CONFORMANCE OF THE STRUCTURAL DRAWINGS AND DOES NOT ALLEVIATE ANY SPECIAL INSPECTION REQUIREMENTS.

SPECIAL INSPECTIONS AND TESTING

SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM ON SHEETS S004-S005. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SUBMITTALS

SUBMIT SHOP DRAWINGS AND OTHER SUBMITTALS TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ITEMS. IF THE SUBMITTALS DIFFER FROM OR ADD TO THE STRUCTURAL CONTRACT DOCUMENTS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE BY THE SEOR.

FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO CONSTRUCTION.

THE USE OF REPRODUCTIONS OR PHOTOCOPIES OF THE CONTRACT DRAWINGS SHALL NOT BE PERMITTED. WHEN CAD OR REVIT FILES ARE PROVIDED TO THE CONTRACTOR OR SUBCONTRACTORS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR/SUBCONTRACTOR TO REMOVE ALL INFORMATION NOT DIRECTLY RELEVANT TO THE SCOPE OF THE SUBMITTAL AS WELL AS ALL REFERENCES TO OUTSIDE SOURCE FILES.

DELEGATED DESIGN SUBMITTALS SHALL INCLUDE DESIGN DRAWINGS AND CALCULATIONS FOR ITEMS THAT ARE DESIGNED BY OTHERS. DELEGATED DESIGN SUBMITTALS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON ON EVERY DRAWING SHEET AND ON THE CALCULATION COVER SHEET, AND SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION. CALCULATIONS AND DETAILS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC AND AS NOTED UNDER "DESIGN CRITERIA".

SUBMITTALS AND DELEGATED DESIGN SUBMITTALS SHALL INCLUDE THE FOLLOWING:

ITEM	SUBMITTAL	DELEGATED DESIGN SUBMITTAL	COMMENTS
CONCRETE MIX DESIGNS	Х		
CONCRETE REINFORCEMENT	Х		
CONCRETE ANCHORAGES	Х		
STRUCTURAL STEEL	Х		
STEEL FASTENERS	Х		
GLUE-LAMINATED MEMBERS	Х		
CURTAIN WALL, WINDOW WALL AND OTHER		Y	
CLADDING AND GLAZING SYSTEMS		Λ	
LADDERS, METAL STAIRS, AND HANDRAILS		X	
MEPF SYSTEMS ANCHORAGE AND BRACING		X	REF. TABLE NOTE 1

TABLE NOTES:

- 1. THE CONTRACTOR SHALL COORDINATE SEISMIC RESTRAINTS OF MECHANICAL, ELECTRICAL, PLUMBING, AND FIRE SAFETY EQUIPMENT AND ASSOCIATED DISTRIBUTION SYSTEMS WITH THE STRUCTURE. CONNECTIONS TO STRUCTURE AND PROVISIONS FOR SEISMIC MOVEMENTS SHALL CONFORM TO ASCE 7-16 CHAPTER 13, BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE SUBMITTED TO THE ARCHITECT AND SEOR PRIOR TO FABRICATION. FOR RISK CATEGORY III AND IV BUILDINGS, THE SYSTEMS ENGINEER SHALL SPECIFY THE REQUIREMENTS FOR EQUIPMENT SEISMIC CERTIFICATION IN THE DEFERRED SUBMITTAL IN ACCORDANCE WITH OSSC SECTION 1705.12.6 AND ASCE 7-16 SECTION 13.2.
- 2. CONTRACTOR SHALL ENGAGE A PROFESSIONAL ENGINEER TO PREPARE AN ASSESSMENT OF ANY EXCAVATIONS THAT MAY REDUCE THE VERTICAL OR LATERAL SUPPORT OF AN EXISTING FOUNDATION AS REQUIRED BY OSSC SECTION 1803.5.7. THE ASSESSMENT SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND SHALL INCLUDE DETAILS AND SEQUENCING FOR CONSTRUCTION OF ANY UNDERPINNING OR BRACING THAT IS REQUIRED.
- 3. CONTRACTOR SHALL COORDINATE AND SHOW ALL REQUIRED PENETRATIONS, WITH DIMENSIONS FOR MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION, TECHNOLOGY AND OTHER SERVICES ON A SINGLE DRAWING FOR REVIEW AT EACH SLAB/DECK, STRUCTURAL WALL AND/OR BEAM.

CONCRETE MIX DESIGNS

CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD CYLINDER TESTS PER ASTM C39. CONCRETE MIX TO BE DESIGNED AND PROPORTIONED BY THE CONTRACTOR IN ACCORDANCE WITH ACI 318-19 CHAPTER 26. ACI 301-16 CHAPTER 4 AND THE FOLLOWING INFORMATION:

CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH of 4,000 PSI AND A MAXIMUM WATER-CEMENT RATIO OF 0.45.

PORTLAND CEMENT CONTENT MAY BE REPLACED WITH FLY ASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F OR TYPE C, SLAG CEMENT CONFORMING TO ASTM C989, AND SILICA FUME CONFORMING TO ASTM C1240 PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.

A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494 USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS SHALL BE INCORPORATED IN CONCRETE MIX DESIGNS. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494 TYPE F OR G MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 10".

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS ALONG WITH TEST DATA COMPLIANT WITH ACI 301-16 AND ACI 318-19 A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER AND SEOR IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.

CONCRETE REINFORCING STEEL

REINFORCEMENT SHALL BE ASTM A615 OR ASTM A706 GRADE 60 DEFORMED REINFORCING BARS ONLY.

ALL REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE. BARS IN BEAMS AND SLABS SHALL BE SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL OR PLASTIC CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STANDARD PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI MNL-66 "ACI DETAILING MANUAL". SHOP DRAWINGS SHALL INCLUDE ELEVATIONS OF ALL BEAMS, WALLS AND COLUMNS SHOWING BAR LOCATIONS.

REINFORCING BARS SHALL NOT BE BENT OR STRAIGHTENED IN THE FIELD WITHOUT APPROVAL OF THE SEOR.

PROVIDE A MINIMUM OF 20" LAP SPLICE ON ALL #4 BARS AND 24" LAP SPLICE ON ALL #5 BARS.

REINFORCING STEEL SHALL HAVE PROTECTION AND SPACING AS FOLLOWS:

CONCRETE COVER			
USE	CLEAR COVER	MIN. CLEAR SPACING	
CONCRETE EXPOSED TO EARTH OR WEATHER	1-1/2" (#5 AND SMALLER)	2db OR 1"	
CONCRETE CAST AGAINST AND EXPOSED TO EARTH	3"	3db OR 1"	

POST-INSTALLED CONCRETE ANCHORS

POST-INSTALLED CONCRETE ANCHORS SHALL BE THE FOLLOWING PRODUCTS, U.N.O.:

TYPE

EXPANSION

CONCRETE SCR

ADHESIVE ANCHO

ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND PRODUCT EVALUATION REPORTS. EMBEDMENTS SPECIFIED ON DRAWINGS ARE "EFFECTIVE" EMBEDMENTS. REFERENCE MANUFACTURER LITERATURE FOR CORRESPONDING ACTUAL EMBEDMENT DEPTHS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.

REQUESTS FOR ANCHOR SUBSTITUTIONS SHALL BE SUBMITTED TO THE SEOR IN WRITING ALONG WITH EVIDENCE OF EQUAL OR GREATER CAPACITY TO THE SPECIFIED CONNECTION.

INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER AS CERTIFIED THROUGH ACI/CRSI AND IN ACCORDANCE WITH ACI 318-19 SECTION 17.8.2.2. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE SEOR PRIOR TO INSTALLATION.

ALL-THREAD ROD FOR ADHESIVE ANCHORS SHALL CONFORM TO ASTM F1554 GRADE 55, U.N.O. ANCHORS EXPOSED TO EARTH OR WEATHER SHALL BE PROTECTED FROM CORROSION BY HOT-DIP GALVANIZING OR USE OF STAINLESS STEEL. PERMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION, U.N.O.

NO LOADS OR WELDS SHALL BE PLACED ON EMBEDDED PLATES OR ANGLES FOR A MINIMUM OF 7 DAYS AFTER CASTING. ADHESIVE ANCHORS SHALL NOT BE INSTALLED FOR A MINIMUM OF 21 DAYS AFTER CASTING CONCRETE IN ACCORDANCE WITH ACI 318-14 SECTION 17.1.2.

STRUCTURAL STEEL SHALL BE OF THE MATERIAL AND TYPE LISTED BELOW, U.N.O.:

STRUCTURAL STEEL			
SHAPE	MATERIAL GRADE		
WIDE FLANGE SHAPES	ASTM A992, GRADE 50		
HOLLOW STRUCTURAL SECTIONS (RECTANGULAR)	ASTM A500, GRADE C (Fy=50KSI)		
PLATES WHERE NOTED	ASTM A572, GRADE 50		
CHANNELS, PLATES AND ANGLES, U.N.O.	ASTM A36		

DESIGN, DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC 360, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

BOLTS SHALL CONFORM TO THE ASTM AND RCSC SPECIFICATIONS FOR JOINTS USING HIGH STRENGTH BOLTS. BOLTS SHALL BE ASTM F3125 GRADE A325 AND GRADE A490 WHERE NOTED, AND SNUG-TIGHT UNLESS NOTED OTHERWISE.

WELDING SHALL CONFORM TO THE AWS CODES FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE STRUCTURAL ENGINEER. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER-METAL MANUFACTURER.

WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM, UNLESS OTHERWISE NOTED. WELDING SHALL BE BY AWS CERTIFIED WELDERS.

NON-SHRINK GROUT USED UNDER BEARING AND BASE PLATES SHALL BE ASTM C 1107, FACTORY-PACKAGED, NONMETALLIC AGGREGATE GROUT, NONCORROSIVE, NONSTAINING, MIXED WITH WATER TO CONSISTENCY SUITABLE FOR APPLICATION AND A 30-MINUTE WORKING TIME. GROUT STRENGTH SHALL BE 8,000 PSI MINIMUM AT 28 DAYS.

GALVANIZING AND DUPLEX COATING

ALL STEEL EXPOSED TO WEATHER OR LOCATED OUTSIDE THE BUILDING ENVELOPE SHALL BE HOT-DIP GALVANIZED UNLESS NOTED OTHERWISE IN PROJECT SPECIFICATIONS OR DRAWINGS. WHERE THESE ELEMENTS ARE ALSO EXPOSED TO VIEW THEY SHALL ADDITIONALLY BE PAINTED OR POWDER COATED PER SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

CONTRACTOR TO COMMUNICATE WITH GALVANIZER FOR THE PROJECT EARLY ON TO INFORM THE GALVANIZER THAT THE STEEL IS TO RECEIVE A DUPLEX COATING. HOT DIPPED GALVANIZED STEEL THAT IS TO BE PAINTED SHALL BE PREPARED PER ASTM D6386. HOT DIPPED GALVANIZED STEEL THAT IS TO BE POWDER COATED SHALL BE PREPARED PER ASTM D7803.

ALL GALVANIZED STEEL IS TO BE DETAILED TO BE SHOP WELDED AND FIELD BOLTED. WHERE FIELD WELDING IS REQUIRED DUE TO FIELD CONDITIONS, REPAIR DAMAGED GALVANIZED COATING WITH ZINC RICH PAINT PER ASTM A780 WITH EFFECTIVE THICKNESS EQUAL TO HOT-DIP GALVANIZED COATING.

	APPROVED ANCHORS	
	HILTI KWIK BOLT TZ2 (ICC ESR-4266) HILTI KWIK BOLT 1 (IAPMO ER-678) SIMPSON STRONG-BOLT 2 (ICC ESR-3037) DEWALT POWER-STUD+ SD2 (ICC ESR-2502)	
EW	HILTI KH-EZ (ICC ESR-3027) SIMPSON TITEN HD (ICC ESR-2713) DEWALT SCREW-BOLT+ (ICC ESR-3889)	
DRS	HILTI HIT-HY 200 V3 (ICC ESR-4868) HILTI HIT-RE 500 V3 (ICC ESR-3814) SIMPSON SET-XP (ICC ESR-2508) SIMPSON SET-3G (ICC ESR-4057) DEWALT PURE110+ (ICC ESR-3298)	

STRUCTURAL STEEL

STRUCTURAL STEEL

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REVISIONS TO THIS SHEET REV. DATE DESCRIPTION

SET ISSUE	DATE		
BP	2024-02-13		
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PROJECT TRACKING					
RBA #:	2327				
P.I.C:	BJ				
PM / PA:	PK/SL				

OSU FRC

Project Name AZALEA EARLY CHILDHOOD CENTER

Project Address 1050 SW MADISON AVE, CORVALLIS OR 97333

GENERAL STRUCTURAL NOTES

SAWN LUMBER

SAWN LUMBER SHALL CONFORM TO THE REQUIREMENTS AS INDICATED IN THE CURRENTLY ACCEPTED NATIONAL DESIGN SPECIFICATION (NDS) DESIGN VALUES FOR WOOD CONSTRUCTION AND CONFORMING TO THE WEST COAST LUMBER INSPECTION BUREAU OR WESTERN WOOD PRODUCTS ASSOCIATION GRADING RULES. LUMBER SHALL BE THE SPECIES, GRADE, AND MOISTURE CONTENT NOTED BELOW, U.N.O.:

USE	SPECIES AND GRADE	MOISTURE CONTENT
LUMBER 2" TO 4" THICK x 5" OR WIDER (JOISTS/RAFTERS)	DOUGLAS FIR-LARCH NO. 2 & BTR	MC 15, KD
LUMBER 2" TO 3" THICK x 4" TO 6" WIDE (STUDS)	DOUGLAS FIR-LARCH STUD	S-DRY, MC 15, KD
LUMBER 5x5 AND GREATER (BEAMS)	DOUGLAS FIR-LARCH NO. 1	MC 15, KD, S-DRY
LUMBER 5x5 AND GREATER (POSTS)	DOUGLAS FIR-LARCH NO. 1	S-DRY
T&G DECKING	DOUGLAS FIR-LARCH COMMERCIAL DEX	S-DRY, MC 15, KD

ALL LUMBER IN CONTACT WITH CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED, UNLESS AN APPROVED MOISTURE BARRIER IS PROVIDED.

CUTTING AND NOTCHING OF JOISTS AND STUDS SHALL CONFORM TO THE TYPICAL WOOD DETAILS PROVIDED OR OSSC SECTIONS 2308.4.2.4, 2308.5.9 AND 2308.7.4 WHERE NO DETAILS ARE SPECIFIED.

SALVAGED LUMBER IS ACCEPTABLE PROVIDED IT IS GRADED BY AN APPROVED GRADING AGENCY PRIOR TO USE AND MEETS A MINIMUM ALLOWABLE BENDING STRESS (Fb) OF 1,000 PSI. CONTRACTOR TO SUBMIT A GRADING REPORT ON EACH MEMBER TO THE ARCHITECT PRIOR TO INSTALLATION.

LUMBER FASTENERS AND ACCESSORIES

FRAMING ACCESSORIES INDICATED SHALL BE MANUFACTURED BY SIMPSON STRONG TIE (OR APPROVED EQUAL) AND OF THE SIZE AND TYPE SPECIFIED. ALL NAIL HOLES SHALL BE FILLED WITH STRUCTURAL FASTENERS, UNLESS NOTED OTHERWISE ON THE DRAWINGS AND FASTENERS SHALL BE INSTALLED FOLLOWING ALL MANUFACTURERS REQUIREMENTS. ACCESSORIES SHALL BE GALVANIZED UNLESS INDICATED OTHERWISE. PROVIDE G90 COATING EXCEPT WHERE IN CONTACT WITH PRESERVATIVE OR FIRE RETARDANT TREATED WOOD IN WHICH CASE G185 SHALL BE PROVIDED. SUBMIT SUBSTITUTION REQUESTS TO ARCHITECT FOR APPROVAL OUTLINING THE FRAMING ACCESSORIES BEING REPLACED AND THE SUBSTITUTED FRAMING ACCESSORIES. ALLOWABLE LOADS FOR THE SPECIFIED ACCESSORIES SHALL BE TABULATED ALONG WITH THE ALLOWABLE LOADS FOR THE SUBSTITUTED ACCESSORIES. SUBSTITUTION REQUESTS WILL ONLY BE APPROVED WHERE SUBSTITUTED PRODUCTS ARE CLEARLY DOCUMENTED TO HAVE EQUAL OR GREATER CAPACITY IN ALL DIRECTIONS.

ALL FRAMING NAILS SHALL BE THE SIZE AND QUANTITY INDICATED AND CONFORM TO ASTM F 1667, INCLUDING SUPPLEMENT 1, "STANDARD SPECIFICATION OF DRIVEN FASTENERS: NAILS, SPIKES, AND STAPLES" AND ICC-ES REPORT ESR-1539 "POWER-DRIVEN STAPLES AND NAILS". NAILS SHALL BE IDENTIFIED BY LABELS (ATTACHED TO THEIR CONTAINERS) THAT SHOW THE MANUFACTURER'S NAME AND ICC-ES REPORT NUMBER, NAIL SHANK DIAMETER AND LENGTH AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FRAMING. NAILING NOT SHOWN SHALL BE AS INDICATED IN OSSC TABLE 2304.10.1 OR ICC ESR-1539. THE FOLLOWING NAIL SIZES SHALL BE USED WITH THE NAIL LENGTH DETERMINED BY MINIMUM PENETRATION INTO FRAMING MEMBER:

FRAMING NAILS							
NAIL TYPE	MINIMUM PENETRATION INTO FRAMING MEMBER (IN.)						
6d	0.113	1.125					
8d	0.131	1.375					
10d	0.148	1.5					
12d	0.148	1.5					
16d	0.148, 0.162	1.5, 1.625					

BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. ALL BOLTS AND LAG SCREWS SHALL BE INSTALLED WITH STANDARD CUT WASHERS.

WOOD STRUCTURAL PANELS

THE TERM "WOOD STRUCTURAL PANEL" REFERS TO A WOOD-BASED PANEL PRODUCT BONDED WITH A WATERPROOF ADHESIVE INCLUDING BOTH PLYWOOD AND ORIENTED STRAND BOARD (OSB). WOOD STRUCTURAL PANELS SHALL CONFORM TO U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARDS PS1 OR PS2 FOR WOOD-BASED STRUCTURAL USE PANELS, OR APA PERFORMANCE STANDARD PRP-108 (ICC-ES ESR-2586). PANELS SHALL BE APA RATED SHEATHING OR APA RATED STURD-I-FLOOR, EXTERIOR OR EXPOSURE 1, OF THE THICKNESS AND SPAN RATING SHOWN ON THE DRAWINGS. PANELS SHALL BE STAMPED WITH THE APA TRADEMARK.

WOOD STRUCTURAL PANEL INSTALLATION SHALL BE IN CONFORMANCE WITH APA RECOMMENDATIONS. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES, UNLESS OTHERWISE INDICATED OR RECOMMENDED BY THE PANEL MANUFACTURER.

ALL ROOF SHEATHING AND FLOOR SHEATHING SHALL BE INSTALLED WITH FACE GRAIN OR STRENGTH AXIS PERPENDICULAR TO SUPPORTS, EXCEPT AS INDICATED ON THE DRAWINGS. ROOF SHEATHING SHALL EITHER BE BLOCKED, TONGUE-AND-GROOVE, OR HAVE EDGES SUPPORTED BY PLYCLIPS. WHERE BLOCKING IS SPECIFICALLY INDICATED ON THE DRAWINGS, T&G EDGES OR PLYCLIPS MAY NOT BE SUBSTITUTED. SHEATHING SHALL BE UNBLOCKED, EXCEPT AS INDICATED ON DRAWINGS. FLOOR SHEATHING SHALL BE FIELD GLUED TO THE FRAMING USING ADHESIVES MEETING APA SPECIFICATION AFG-01 OR ASTM D3498. TONGUE AND GROOVE PANELS SHALL ALSO BE GLUED AT THE T&G JOINT.

SHEAR WALL SHEATHING SHALL BE INSTALLED EITHER HORIZONTALLY OR VERTICALLY AND BE BLOCKED WITH 2x FRAMING AT ALL PANEL EDGES. NAILING NOT SHOWN SHALL BE AS INDICATED IN OSSC TABLE 2304.10.1.

GLUED-LAMINATED MEMBERS

GLUED-LAMINATED (GLULAM) MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH CURRENT ANSI STANDARD A190.1, AMERICAN NATIONAL STANDARD FOR STRUCTURAL GLUED LAMINATED TIMBER OR OTHER CODE- APPROVED DESIGN, MANUFACTURING AND/OR QUALITY ASSURANCE PROCEDURES. EACH MEMBER SHALL BEAR AN AITC OR APA-EWS IDENTIFICATION MARK OR BE ACCOMPANIED BY A CERTIFICATE OF CONFORMANCE. APA-EWS MARKS TO BE PLACED ON SURFACES NOT EXPOSED IN COMPLETED CONSTRUCTION. ONE COAT OF END SEALER SHALL BE APPLIED IMMEDIATELY AFTER TRIMMING IN EITHER THE SHOP OR IN THE FIELD.

GLULAM MEMBERS SHALL BE INDUSTRIAL (HIDDEN) APPEARANCE CLASSIFICATION, REFERENCE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

GLULAM MEMBERS SHALL BE OF MINIMUM ALLOWABLE DESIGN PROPERTIES AS ESTABLISHED BY ASTM D3737:

GLUED-LAMINATED BEAMS							
COMBINATION SYMBOL (SPECIES)	FLEXURAL STRESS, Fb (PSI)	HORIZONTAL SHEAR STRESS Fv (PSI)	COMPRESSION STRESS PERP TO GRAIN Fc,perp (PSI)	MODULUS OF ELASTICITY (PSI)			
24F-V4 (DF/DF) (SIMPLE SPAN)	+2,400 / -1,850	265	650	1,800,000			
24F-V8 (DF/DF) (CONTINUOUS OR CANTILEVER)	2,400	265	650	1,800,000			

REFERENCE SPECIFICATIONS FOR FABRICATION AND MILLING TOLERANCES FOR TIMBER SIZES, HOLES, AND CONNECTIONS. CONNECTIONS SHALL BE SHOP FABRICATED TO GREATEST EXTENT INCLUDING CUTTING TO LENGTH AND DRILLING HOLES.

NOTCHES, DAPS, HOLES, ETC. SHALL BE REPRESENTED ON SHOP DRAWINGS FOR REVIEW BY SEOR. FIELD NOTCHING AND BORING OF GLULAM MEMBERS IS NOT ALLOWED UNLESS APPROVED BY SEOR.

GLULAM PRODUCTS SHALL CONTAIN AVERAGE MOISTURE CONTENT OF 16% OR LESS AT TIME OF MANUFACTURE. REFERENCE SPECIFICATIONS FOR ALLOWED DIMENSIONAL TOLERANCES AT TIME OF MANUFACTURE.

3x TONGUE-AND-GROOVE DECKING

TONGUE-AND-GROOVE DECK SHALL SPAN BETWEEN SUPPORTS. FOR 2-SPAN CONDITION, DECKING MAY HAVE A JOINT ALIGNED OVER THE MIDDLE SUPPORT. WHERE DECKING OVERHANGS A SUPPORT, NO JOINT IS PERMITTED.

DECKING SHALL BE INSTALLED WITH TONGUES UP ON SLOPED OR PITCHED ROOFS AND WITH PATTERN FACES DOWN. EACH PIECE SHALL BE TOE NAILED THROUGH THE TONGUE AT EACH SUPPORT WITH ONE 40d COMMON NAIL AND FACE NAILED AT EACH SUPPORT WITH ONE 60d COMMON NAIL. COURSES SHALL BE SPIKED TO EACH OTHER WITH 8 INCH SPIKES AT INTERVALS NOT EXCEEDING 30 INCHES THROUGH PREDRILLED EDGE HOLES AND WITH ONE SPIKE AT A DISTANCE NOT EXCEEDING 10 INCHES FROM EACH END OF EACH PIECE.



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OSU FRC

Project Name AZALEA EARLY CHILDHOOD CENTER

Project Address 1050 SW MADISON AVE, CORVALLIS OR 97333

GENERAL STRUCTURAL NOTES

STATEMENT OF SPECIAL INSPECTION NOTES: SPECIAL INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2019 OSSC, CONTRACT DOCUMENTS AND APPROVED 1 REFER TO SPECIAL INSPECTION AND TESTING TABLES FOR PROJECT REQUIREMENTS. 2. SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT THE REQUIREMENTS OF ASTM E329 (MATERIALS). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRU AND ARCHITECT A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE APPROVED BY THE BUIL WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1(1) OF AWS D1.1. THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN TH 3 REPORTS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STR 4. ENGINEER, ARCHITECT, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTIO THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED. QUALITY ASSURANCE (QA) IS REQUIRED FOR STRUCTURAL STEEL ITEMS PER AISC 360 AND 341 UNLESS SPECIFICALLY NO QUALITY CONTROL (QC) TO BE PROVIDED BY THE FABRICATOR, ERECTOR OR OTHER RESPONSIBLE CONTRACTOR AS AF CONTRACTOR AND SPECIAL INSPECTOR TO DOCUMENT QUALITY CONTROL AS REQUIRED IN AISC 360 SECTION N3 AND A **INSPECTION TYPES:**

CONTINUOUS : THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSP PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. PERIODIC : THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROV

INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLE WORK.

OBSERVE : OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OB PERFORM : INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.

- PERFORM INSPECTION PRIOR TO FINAL ACCEPTANCE OF THE ITEM FOR TEN WELDS TO BE MADE BY A GIVEN WELDER, V DEMONSTRATING UNDERSTANDING OF REQUIREMENTS AND POSSESSION OF SKILLS AND TOOLS TO VERIFY THESE ITEM DESIGNATION OF THIS TASK SHALL BE REDUCED TO OBSERVE, AND THE WELDER SHALL PERFORM THIS TASK. SHOULD DETERMINE THAT THE WELDER HAS DISCONTINUED PERFORMANCE OF THIS TASK, THE TASK SHALL BE RETURNED TO F SUCH TIME AS THE INSPECTOR HAS RE-ESTABLISHED ADEQUATE ASSURANCE THAT THE WELDER WILL PERFORM THE IN LISTED.
- SPECIAL INSPECTION OF MECHANICAL POST INSTALLED ANCHORS SHALL BE IN STRICT CONFORMANCE WITH THE ICC RE MANUFACTURER'S INSTALLATION REQUIREMENTS. ANCHOR INSTALLERS SHALL BE QUALIFIED AS REQUIRED BY JURISDI REQUIREMENTS.
- INSPECTION REPORTS SHALL IDENTIFY NAMES OF INSTALLERS.
- SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THAT THE ANG • INSPECTED PER APPROVED ANCHOR EVALUATION REPORT.

TESTING ABBREVIATIONS: 9

NDT - NON-DESTRUCTIVE TESTING

	GENERAL - SPECIAL INSPECTIONS							
	OSSC CODE	CODE OR	FREQUENCY (NOTE 6)					
STSTEM OR MATERIAL	REFERENCE	REFERENCE	CONTINUOUS	PERIODIC				
FABRICATORS	1705.10 1704.2.5							
DEFERRED SUBMITTALS				х				
SUBMITTALS TO THE BUILDING OFFICIAL	1704.5			х				
POST INSTALLED MECHANICAL ANCHORS AND ADHESIVE ANCHORS (EXCLUDING CONDITIONS NOTED ABOVE) IN HARDENED CONCRETE AND COMPLETED MASONRY				х				

	SOILS/GEOTECHNICAL - SPECIAL INSPECTIONS							
SYSTEM OR MATERIAL	OSSC CODE		FREQUENCY (NOTE 6)					
	REFERENCE	REFERENCE	CONTINUOUS	PERIODIC				
		SOILS						
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY				х				
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL				х				
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	1705.6	GEOTECHNICAL		х				
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL		REPORT	х					
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY				х				

SOIL SUGEOTECHNICAL - TESTING

		CODE OR	FREQUENCY (NOTE 6)					
SYSTEM OR MATERIAL	REFERENCE	STANDARD REFERENCE	CONTINUOUS	PERIODIC	REMARKS			
FILL IN-PLACE DENSITY OR PREPARED SUBGRADE DENSITY	1705.6	VARIES; GEOTECHNICAL REPORT OR MINIMUM PER OSSC APPENDIX J107.5, WHICHEVER IS GREATER		х	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR			
MATERIAL VERIFICATION		VARIES; CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS		х	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR			
TEST ELEMENTS	1705.6 1705.7		REFERENCE SPE FOR PERFOR VARIFICATION AND TESTING REQU	CIFICATIONS RMANCE) PROOF LOAD IIREMENTS	BY THE GEOTECHNICAL ENGINEER			

CONCRETE - SPECIAL INSPECTIONS							
		CODE OR STANDARD REFERENCE	FREQUENCY	(NOTE 6)			
SYSTEM OR MATERIAL	REFERENCE		CONTINUOUS	PERIODIC	REMARKS		
GENERAL	1705.3 1901.6	ACI 318: 26.13			SPECIAL INSPECTIONS OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1705.3 OF THE IBC AND SECTION 26.13 OF ACI 318.		
REINFORCING STEELPLACEMENT	1901.5.2	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3, 26.13.3.3		x	REINFORCING TO COMPLY WITH ALL CODE PROTECTION, SPACING AND TOLERANCE LIMITS.		
INSPECT ANCHORS/BOLTS CAST IN CONCRETE	-	ACI 318: 17.8.2	x	x	ALL CAST-IN-PLACE ANCHORS/BOLTS SHALL BE VISUALLY INSPECTED. REFERENCE STEEL INSPECTIONS FOR ADDITIONAL INSTALLATION, MATERIAL AND WELDING INSPECTIONS OF STEEL ITEMS EMBEDDED IN CONCRETE (HEADED STUDS, DBA's, ETC.)		
VERIFYING USE OF REQUIRED MIX DESIGN(S)	1904.1 1904.2 1908.2 1908.3	ACI 318: CH. 19, 26.4.3, 26.4.4		x			
CONCRETE SPECIMENS FOR TESTING	1908.10	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	х		PRIOR TO CONCRETE PLACEMENT, FABRICATE CONCRETE SPECIMENS FOR TESTING. SEE THE CONCRETE TESTING TABLE FOR ADDITIONAL INFORMATION.		
CONCRETE PLACEMENT	1908.6, 1908.7, 1908.8	ACI 318: 26.5, 26.13.3.2(a)	х				
CONCRETE CURING	1908.9	ACI 318: 26.5.3 - 26.5.5, 26.13.3.3		x	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURES AND TECHNIQUES		
VERIFICATION OF FORMWORK		ACI 318: 26.11.1.2(b), 26.13.3.3		x	SPECIAL INSPECTIONS APPLY TO SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		
EMBEDDED ITEMS IN CONCRETE				x	ALL NON-STRUCTURAL EMBEDDED ITEMS, SUCH AS CONDUITS, PIPES AND SLEEVES, SHALL BE REVIEWED FOR CONFORMANCE WITH STRUCTURAL DOCUMENTS FOR SIZE, SPACING, LOCATION, EDGE DISTANCE AND TRIM REINFORCING.		

CONCRETE - TESTING							
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)	REMARKS			
CONCRETE STRENGTH	1705.3	ASTM C39					
CONCRETE SLUMP	ASTM C172	ASTM C143	EACH 150 CY NOR LESS THAN	FABRICATE SPECIMENS AT TIME FRESH CONCRETE			
CONCRETE AIR CONTENT	ACI 318 26.12	ASTM C231	WALL PLACED EACH SHIFT	IS PLACED			
CONCRETE TEMPERATURE	ACI 318 26.5	ASTM C1064					

S	SUBMITTALS.
L 7 7 7 7	AGENCY MEETING CTURAL ENGINEER DING OFFICIAL.
O HE	N DOCUMENTS. E INSPECTION
וא ר יוכ	JCTURAL STATING THAT THE N DOCUMENTS AND
IC PF	OTED OTHERWISE. PLICABLE. SC 341 SECTION J2.
Ρ	ECTOR WHO IS
E	D SPECIAL TION OF THE
35	ERVATIONS.
	ITH THE WELDER 6, THE PERFORM HE INSPECTOR ERFORM UNTIL SPECTION TASKS
El C	PORT AND TION
С	HORS WERE
	REMARKS
	SPECIAL INSPECTION IS REQUIRED FOR STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES FABRICATED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTIONS SHALL BE PERFORMED DURING FABRICATION. PERFORMING SPECIAL INSPECTIONS IS NOT REQUIRED, WHERE FABRICATOR HAS BEEN APPROVED AS AN APPROVED FABRICATOR, PER SECTION 1704.2.5.1.
-	SPECIAL INSPECTION REQUIREMENTS FOR DEFERRED SUBMITTAL ITEMS, INCLUDING REQUIREMENTS FOR DESIGNATED SEISMIC SYSTEMS IN ACCORDANCE WITH OSSC SECTION 1705.12.4 IF APPLICABLE, TO BE SPECIFIED BY THE SYSTEM ENGINEER AND INCLUDED WITH DEFERRE SUBMITAL DOCUMENTS.
	CERTIFICATES OF COMPLIANCE, REPORTS OF PRE- CONSTRUCTION TESTS, OR REPORTS OF MATERIAL PROPERTIES SHALL BE SUBMITTED TO THE BUILDING OFFICIAL.

REMARKS

BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR



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PROJECT TRACKING		
RBA #:	2327	
P.I.C:	BJ	
PM / PA:	PK/SL	

Owner OSU FRC

Project Name AZALEA EARLY CHILDHOOD CENTER

Project Address 1050 SW MADISON AVE, CORVALLIS OR 97333

SPECIAL INSPECTIONS

SYSTEM	OR	MATER

CONTRACTOR QUALITY CONTROL REQ

STEEL FABRICATION

FABRICATION OF STRUCTURAL ELEN

MATERIAL VERIFICATION OF STRUCT

MATERIAL VERIFICATION OF ANCHOR B THREADED RODS

MATERIAL VERIFICATION OF WELD FILL

STRUCTURAL STEEL WELDING

VERIFYING USE OF PROPER WPS'S VERIFYING WELDER QUALIFICATIONS

SINGLE PASS FILLET WELDS GREATER

SINGLE PASS FILLET WELDS LESS THA

WELDING STAIR AND RAILING SYSTEMS

VERIFICATION OF JOINT & CONNECTION MEMBER AND COMPONENT LOCATIONS, STIFFENERS

STEEL - SPECIAL INSPECTIONS					
	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	INSPECTION (NOTES 5 AND 6)		
IAL			CONTINUOUS/ PERFORM	PERIODIC/ OBSERVE	REMARKS
UIREMENTS		AISC 360 CHAPTER N	х	Х	CONTRACTOR TO PROVIDE QUALITY CONTROL FOR ALL ITEMS INDICATED TO BE OBSERVED AND/OR PERFORMED IN TABLE BELOW
/ENTS	1704.2.5.1	AISC 360		Х	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
TURAL STEEL	1505.2.1 2203.1 TABLE 1705.2	ASTM A6 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS AISC 360 A3.1 AISC 360 N3.2		Х	CERTIFIED MILL TEST REPORTS
3OLTS AND		AISC 360 A3.4 AISC 360 N3.2 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS		х	MANUFACTURER'S CERTIFIED TEST REPORTS
-ER METALS	1705.2.1.1 TABLE 1705.2-5	AISC 360 A3.5 AISC 360 N3.2 APPLICABLE AWS A5 DOCUMENTS		Х	MANUFACTURER'S CERTIFIED TEST REPORTS
	1705.2.1 AWS D1.1	AISC 360 N3.2			RETAIN A RECORD OF WELDING PROCEDURE SPECIFICATIONS
		AWS D1.1		Х	RETAIN A RECORD OF QUALIFICATION CARDS
2 THAN 5/16"		ABLE 1705.2-6 AWS D1.1 CLAUSE	Х		ALL WELDS VISUALLY INSPECTED PER AWS D1.16.9
N OR EQUAL TO 5/16"	TABLE 1703.2-0			Х	
S	1705.2(2.5)	AWS D1.1 CLAUSE 6		X	ALL WELDS VISUALLY INSPECTED PER AWS D1.1 6.9
N DETAILS INCLUDING S, BRACING, AND	TABLE 1705.2-7	AWS D1.1		Х	

S-005

SPECIAL INSPECTIONS

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Owner OSU FRC

Project Name AZALEA EARLY CHILDHOOD CENTER

Project Address 1050 SW MADISON AVE, CORVALLIS OR 97333

FOUNDATION PLAN

S-121



NOTES:

NOTE	<u>:S:</u>	
1.	(E)	INDICATES EXISTING.
2.	(N)	INDICATES NEW STRICTURE.
3. –		INDICATES EXISTING STRUCTURE.
4.	X -XX.XX'	INDICATES FOOTING TYPE. REF. SCHEDULE. 1/S-501 INDICATES TOP OF FOOTING ELEVATION.
5.		CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, AND ELEVATIONS PRIOR TO FABRICATION AND ERECTION. NOTIFY ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM THOSE SHOWN ON THE DRAWINGS.
9.		CONTRACTOR TO SHORE ALL EXISTING FRAMING AS REQUIRED FOR DEMOLITION AND RE-FRAMING WORK.
7.		ALL EXPOSED FRAMING SHALL BE INSPECTED FOR CRACKS AND DAMAGE BY THE CONTRACTOR AND FINDINGS REPORTED TO THE ARCHITECT.



1) 2ND FLOOR FRAMING PLAN



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2nd FLOOR FRAMING PLAN

S-122



INDICATES EXISTING.

NOTES:

9.

1. (E)

INDICATES NEW STRUCTURE.

INDICATES ABOVE STRUCTURE.

INDICATES BELOW STRUCTURE.

INDICATES EXISTING STRUCTURE.

ALL EXISTING HALLWAY WALLS ARE BEARING WALLS. REF. DEMO PLANS ON SHEET A100 FOR WALLS BEING REMOVED.

PROVIDE TEMPORARY SHOWING WHILE FINAL STRUCTURE IS BEING INSTALLED.

REF. 2/S-601 FOR TYPICAL OPENING CONSTRUCTION AND THE SIZES OF ALL HEADERS NOT IDENTIFIED ON THE PLANS. ALL HEADERS SHALL BEAR ON A MINIMUM OF ONE 2x TRIMMER STUD U.N.O. REF. ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS TYP.

WHERE (E) 2ND FLOOR GL. BEAMS ARE NOTED TO BE STRENGTHENED, THIS WORK IS BEING PERFORMED IN GROUND FLOOR CEILING.







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2ND FLOOR REFLECTED CEILING PLAN



INDICATES EXISTING.

INDICATES NEW STRUCTURE.

INDICATES ABOVE STRUCTURE.

INDICATES BELOW STRUCTURE.

INDICATES EXISTING STRUCTURE.

ALL EXISTING HALLWAY WALLS ARE BEARING WALLS. REF. DEMO PLANS ON SHEET A100 FOR WALLS BEING REMOVED.

PROVIDE TEMPORARY SHOWING WHILE FINAL STRUCTURE IS BEING INSTALLED.

REF. 2/S-601 FOR TYPICAL OPENING CONSTRUCTION AND THE SIZES OF ALL HEADERS NOT IDENTIFIED ON THE PLANS. ALL HEADERS SHALL BEAR ON A MINIMUM OF ONE 2x TRIMMER STUD U.N.O. REF. ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS TYP.



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Owner OSU FRC

Project Name AZALEA EARLY CHILDHOOD CENTER

Project Address 1050 SW MADISON AVE, CORVALLIS OR 97333

CONCRETE DETAILS















NEW WOOD POST -

REMOVE SHEATHING -

LOCALLY. WOO POST TO REBAR

ON (E) HEADER

(E) FLOOR JOIST V.I.F. TYP.

NEW BLOCK

REF. PLAN

<u>NOTE:</u> JOIN MULTIPLE STUDS WITH (2) ROWS 10d NAILS STAGGERED @ 12" o.c. TYP.





1203 Willamette Street Suite 210 Eugene, Oregon 97401 541 485 1003 rowellbrokaw.com

Architecture. Design. Strategy.





REVISIONS TO THIS SHEET REV. DATE DESCRIPTION

SET ISSUE	DATE
BP	2024-02-13
100% DD	2023-12-15
100% SD	2023-10-28

PROJECT TRACKING		
RBA #:	2327	
P.I.C:	BJ	
PM / PA:	PK/SL	

Owner OSU FRC

- SIMPSON BC4

(E) SHEATHING

- (E) HEADER FIELD VERIFY

- (E) WOOD POST

RÉF. PLAN

TYP.

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CORVALLIS OR 97333

DETAILS

S-601

(3) NEW POST AT (E) JOIST







CEILING JOIST AT GL BEAM

1









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